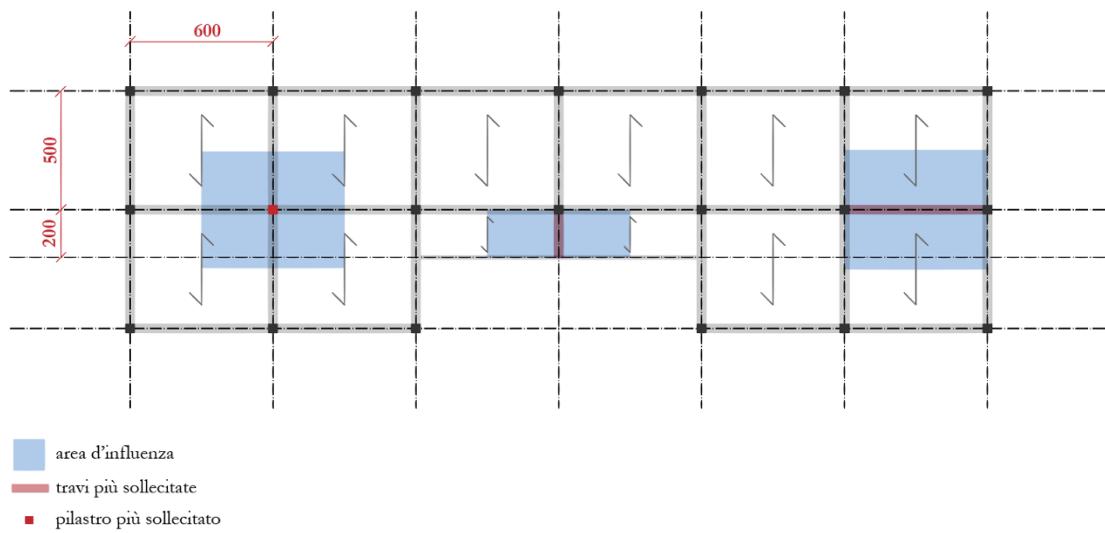
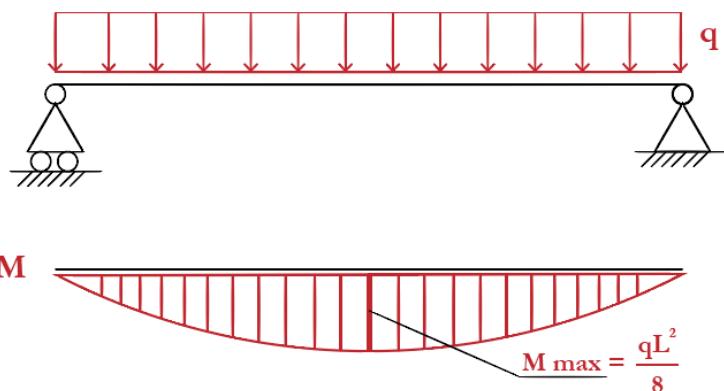


## Allegato 01 \_ Progetto di una struttura a telai piani in calcestruzzo armato

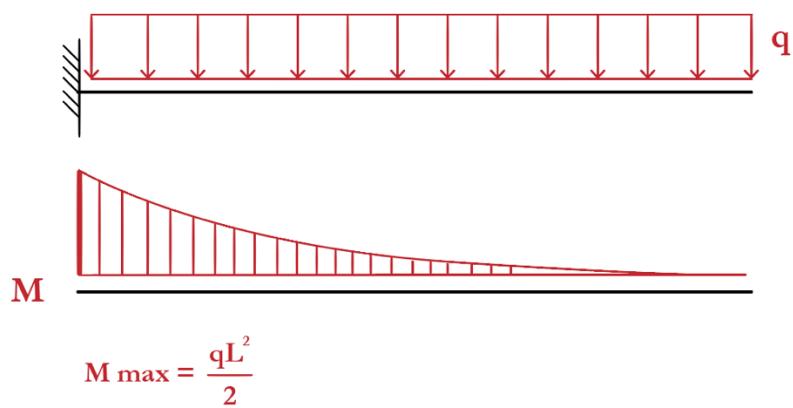
### ▼ IMG. 01



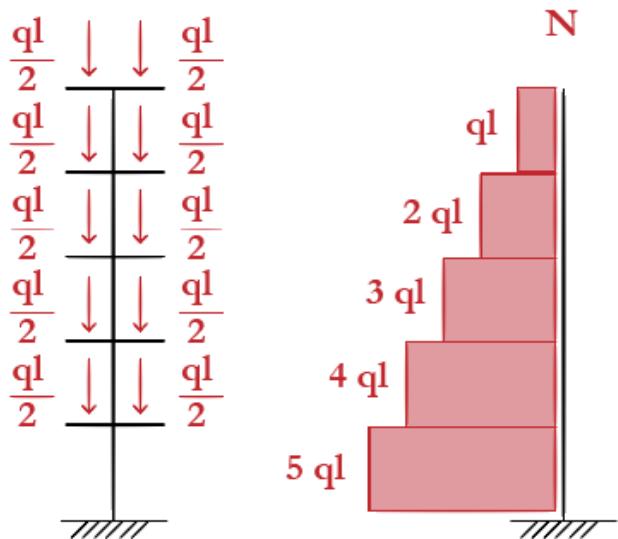
### ▼ IMG. 02



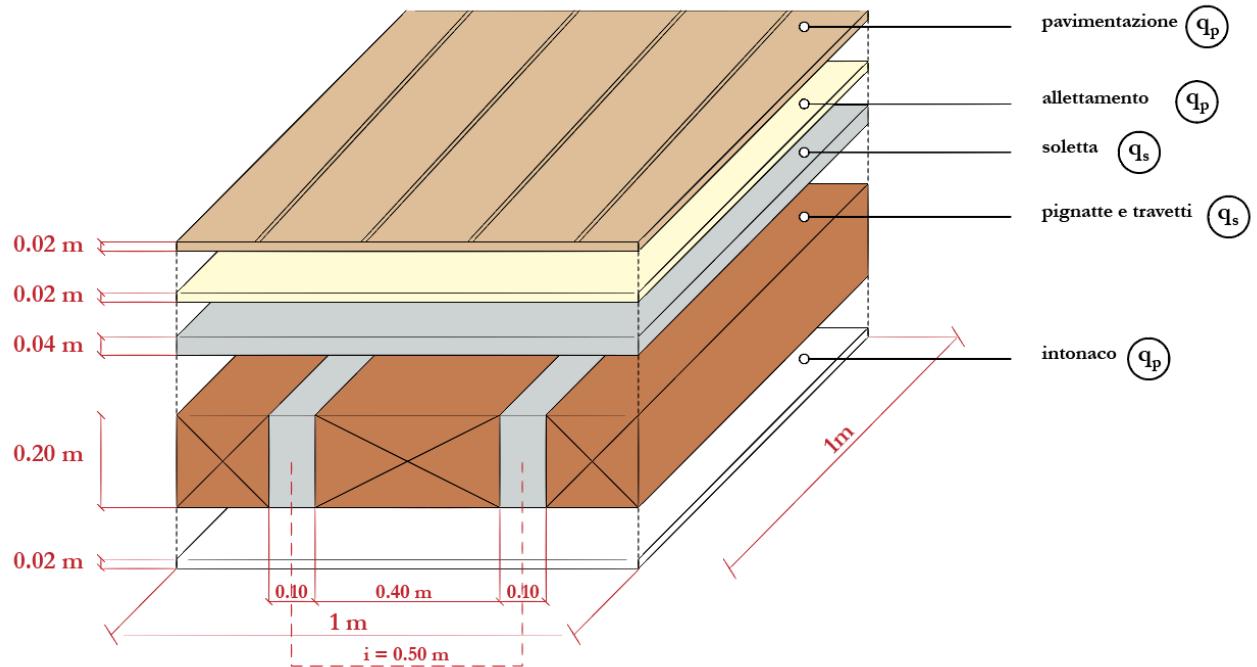
### ▼ IMG. 03



▼ IMG. 04



▼ IMG. 05



▼ **IMG. 06**

(q<sub>s</sub>)

**CARICHI STRUTTURALI tot. 3,76 ( $\frac{KN}{m^2}$ )**

- **Soletta)** 
$$\frac{[0.04 \text{ (m)} \times 1.0 \text{ (m)} \times 1.0 \text{ (m)}] \cdot 25 \left(\frac{KN}{m^2}\right)}{1.0 \text{ (m}^2\text{)}} = 1 \text{ KN/m}^2$$
- **Travetti)** 
$$\frac{[0.10 \text{ (m)} \times 1.0 \text{ (m)} \times 0.20 \text{ (m)}] \cdot 25 \left(\frac{KN}{m^2}\right) \cdot \frac{1}{0.5 \text{ m}}}{1.0 \text{ (m}^2\text{)}} = 1 \text{ KN/m}^2$$
- **Pignatte)** 
$$\frac{[0.40 \text{ (m)} \times 1.0 \text{ (m)} \times 0.20 \text{ (m)}] \cdot 25 \left(\frac{KN}{m^2}\right) \cdot \frac{1}{0.5 \text{ m}}}{1.0 \text{ (m}^2\text{)}} = 1,76 \text{ KN/m}^2$$

(q<sub>p</sub>)

**SOVRACCARICHI PERMANENTI tot. 1,58 ( $\frac{KN}{m^2}$ )**

- **Pavimentazione)** 
$$\frac{[1.0 \text{ (m)} \times 1.0 \text{ (m)} \times 0.02 \text{ (m)}] \cdot 5 \left(\frac{KN}{m^3}\right)}{1.0 \text{ (m}^2\text{)}} = 0,1 \text{ KN/m}^2$$
- **Allettamento)** 
$$\frac{[1.4 \text{ (m)} \times 1.0 \text{ (m)} \times 0.02 \text{ (m)}] \cdot 20 \left(\frac{KN}{m^3}\right)}{1.0 \text{ (m}^2\text{)}} = 0,4 \text{ KN/m}^2$$
- **Massetto)** 
$$\frac{[1.0 \text{ (m)} \times 1.0 \text{ (m)} \times 0.04 \text{ (m)}] \cdot 18 \left(\frac{KN}{m^3}\right)}{1.0 \text{ (m}^2\text{)}} = 0,72 \text{ KN/m}^2$$
- **Intonaco)** 
$$\frac{[1.0 \text{ (m)} \times 1.0 \text{ (m)} \times 0.02 \text{ (m)}] \cdot 18 \left(\frac{KN}{m^3}\right)}{1.0 \text{ (m}^2\text{)}} = 0,36 \text{ KN/m}^2$$

(q<sub>p</sub>)

**CARICHI ACCIDENTALI tot. 2,00 ( $\frac{KN}{m^2}$ )**

▼ **IMG. 07 – 08 - 09**

$$q_* = 1.3 \times 3.76 \left( \frac{KN}{m^2} \right) + 1.5 \times 1.58 \left( \frac{KN}{m^2} \right) + 1.5 \times 2.0 \left( \frac{KN}{m^2} \right) = 10,26 \left( \frac{KN}{m^2} \right)$$

$$q_u \left( \frac{KN}{m} \right) = q_* \times i = 10,26 \left( \frac{KN}{m^2} \right) \times 5.0 (m) = 51,29 \left( \frac{KN}{m} \right)$$

$$M_{max} = \frac{51,29 \left( \frac{KN}{m} \right) \times 6.0 (m)}{8} = 230,81 (KN \cdot m)$$

interasse (m)	$q_s$ (KN/m <sup>2</sup> )	$q_p$ (KN/m <sup>2</sup> )	$q_a$ (KN/m <sup>2</sup> )	$q_u$ (KN/m)	luce (m)	$M_{max}$ (KN*m)
5,00	3,76	1,58	2,00	51,29	6,00	230,81

▼ **IMG. 10**

**Tabella 4.1.I – Classi di resistenza**

CLASSE DI RESISTENZA
C8/10
C12/15
C16/20
C20/25
C25/30
C28/35
C 32/40
C35/45
C40/50
C45/55
C50/60
C55/67
C60/75
C70/85
C80/95
C90/105

**Tabella 4.1.II – Impiego delle diverse classi di resistenza**

STRUTTURE DI DESTINAZIONE	CLASSE DI RESISTENZA MINIMA
Per strutture non armate o a bassa percentuale di armatura (§ 4.1.11)	C8/10
Per strutture semplicemente armate	C16/20
Per strutture precomprese	C28/35

## ▼ IMG. 11

	B	C	D	E	F	G	H	I	J	K	L	M	N	O	P	Q	R	S	T	U
1	$q_e$ (KN/m <sup>2</sup> )	$q_b$ (KN/m <sup>2</sup> )	$q_u$ (KN/m <sup>2</sup> )	$q_i$ (KN/m)	luce (m)	$M_{max}$ (KN·m)	$f_{vk}$ (N/mm <sup>2</sup> )	$f_{vd}$ (N/mm <sup>2</sup> )	$f_{ck}$ (N/mm <sup>2</sup> )	$f_{cd}$ (N/mm <sup>2</sup> )	$\beta$	$r$	b (cm)	$h_i$ (cm)	$\delta$ (cm)	$H_{min}$ (cm)	H	$H/l$	area (m <sup>2</sup> )	peso unitario (KN/m)
2	3,76	1,58	2,00	51,29	6,00	230,81	450,00	391,30	16,00	9,07	0,26	2,91	30,00	84,85	5,00	89,85	90,00	0,15	0,27	6,75
3																				
4																				

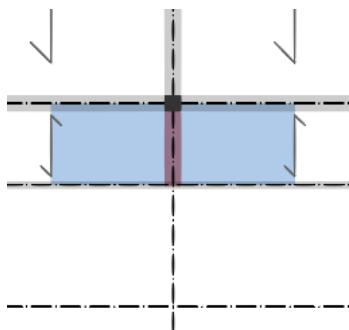
## ▼ IMG. 12

	B	C	D	E	F	G	H	I	J	K	L	M	N	O	P	Q	R	S	T	U
1	$q_e$ (KN/m <sup>2</sup> )	$q_b$ (KN/m <sup>2</sup> )	$q_u$ (KN/m <sup>2</sup> )	$q_i$ (KN/m)	luce (m)	$M_{max}$ (KN·m)	$f_{vk}$ (N/mm <sup>2</sup> )	$f_{vd}$ (N/mm <sup>2</sup> )	$f_{ck}$ (N/mm <sup>2</sup> )	$f_{cd}$ (N/mm <sup>2</sup> )	$\beta$	$r$	b (cm)	$h_i$ (cm)	$\delta$ (cm)	$H_{min}$ (cm)	H	$H/l$	area (m <sup>2</sup> )	peso unitario (KN/m)
2	3,76	1,58	2,00	51,29	6,00	230,81	450,00	391,30	16,00	9,07	0,26	2,91	30,00	84,85	5,00	89,85	90,00	0,15	0,27	6,75
3	3,76	1,58	2,00	51,29	6,00	230,81	450,00	391,30	16,00	9,07	0,26	2,91	30,00	84,85	5,00	89,85	90,00	0,15	0,27	6,75
4																				

## ▼ IMG. 13

	B	C	D	E	F	G	H	I	J	K	L	M	N	O	P	Q	R	S	T	U
1	$q_e$ (KN/m <sup>2</sup> )	$q_b$ (KN/m <sup>2</sup> )	$q_u$ (KN/m <sup>2</sup> )	$q_i$ (KN/m)	luce (m)	$M_{max}$ (KN·m)	$f_{vk}$ (N/mm <sup>2</sup> )	$f_{vd}$ (N/mm <sup>2</sup> )	$f_{ck}$ (N/mm <sup>2</sup> )	$f_{cd}$ (N/mm <sup>2</sup> )	$\beta$	$r$	b (cm)	$h_i$ (cm)	$\delta$ (cm)	$H_{min}$ (cm)	H	$H/l$	area (m <sup>2</sup> )	peso unitario (KN/m)
3,76	1,58	2,00	51,29	6,00	230,81	450,00	391,30	16,00	9,07	0,26	2,91	40,00	73,48	5,00	78,48	90,00	0,13	0,36	9,00	

## ▼ IMG. 14



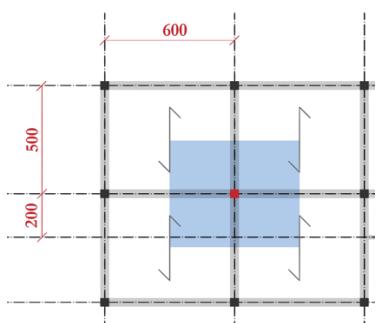
## ▼ IMG. 15

interasse (m)	$q_e$ (kN/mq)	$q_b$ (kN/mq)	$q_u$ (kN/mq)	$q_i$ (kN/m)	luce (m)	$M_{max}$ (kN·m)	$f_{vk}$ (N/mm <sup>2</sup> )	$f_{vd}$ (N/mm <sup>2</sup> )	$f_{ck}$ (N/mm <sup>2</sup> )	$f_{cd}$ (N/mm <sup>2</sup> )	$\beta$	$r$	b (cm)	$h_i$ (cm)	$\delta$ (cm)	$H_{min}$ (cm)	H (cm)	area (m <sup>2</sup> )	peso (kN/m)
6	3,76	1,58	2,00	61,55	2	123,10	450	391,30	16	9,07	0,26	2,91	40	53,66	5	58,66	70	0,28	7,00
				70,65	2,00	141,30	450,00	391,30	16,00	9,07	0,26	2,91	40,00	57,49	5,00	62,49	verificata		

## ▼ IMG. 16

$q_e$	$E$ (N/mm <sup>2</sup> )	$I_x$ (cm <sup>4</sup> )	$v_{max}$ (cm)	$I/v_{max}$	
45,04	21000	1143333	0,04	5330,82	Sì

## ▼ IMG. 17



▼ IMG. 18

$L_p$	$L_s$	Area	$trave_p$	$trave_s$	$q_{trave}$	$q_s$	$q_p$	$q_a$	$q_{solaio}$	$n_{piani}$	N	$f_{ck}$	$f_{cd}$	$A_{min}$
m	m	m <sup>2</sup>	kN/m	kN/m	kN	kN/mq	kN/mq	kN/mq	kN		kN	Mpa	Mpa	cm <sup>2</sup>
6,00	5,00	30,00	9,00	9,00	128,70	3,76	1,58	2,00	307,74	5	2182	16,00	9,1	2406,8

▼ IMG. 19

E	$\beta$	I	$\lambda^*$	$\rho_{min}$	$b_{min}$	b	$h_{min}$	h	$A_{design}$
Mpa		m		cm	cm	cm	cm	cm	cm <sup>2</sup>
21000	1,00	4,00	151,19	2,65	9,16	40,00	60,17	40,00	1600

▼ IMG. 20

E	$\beta$	I	$\lambda^*$	$\rho_{min}$	$b_{min}$	b	$h_{min}$	h	$A_{design}$	$I_{design}$	$I_{max}$	$W_{max}$	$q_t$	M	$\sigma_{max}$
Mpa		m		cm	cm	cm	cm	cm	cm <sup>2</sup>	cm <sup>4</sup>	cm <sup>4</sup>	cm <sup>3</sup>	kN/m	kN*m	Mpa
21000	1,00	4,00	151,19	2,65	9,16	40,00	60,17	40,00	1600	213333	213333	10666,67	51,29	153,87	28,06

▼ IMG. 21

E	$\beta$	I	$\lambda^*$	$\rho_{min}$	$b_{min}$	b	$h_{min}$	h	$A_{design}$	$I_{design}$	$I_{max}$	$W_{max}$	$q_t$	M	$\sigma_{max}$
Mpa		m		cm	cm	cm	cm	cm	cm <sup>2</sup>	cm <sup>4</sup>	cm <sup>4</sup>	cm <sup>3</sup>	kN/m	kN*m	Mpa
21000	1,00	4,00	151,19	2,65	9,16	65,00	37,03	65,00	4225	#####	1487552	45770,83	51,29	153,87	8,53